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**B.Tech. 6<sup>th</sup> Semester End-Term Examination**

**GEOTECHNICAL ENGINEERING - II**

**(New Regulation & New Syllabus)**

Full Marks – 70

Time – Three hours

The figures in the margin indicate full marks  
for the questions.

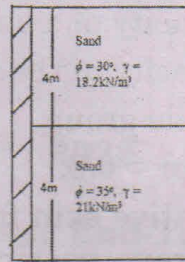
Answer question No. 1 and any *four* from the rest.

1. Answer the following MCQ (10 × 1 = 10)
- (i) The angle of internal friction of a dry cohesionless soil is 30°. The coefficient of earth pressure at rest condition is
- (a) 0.43  
(b) 0.33  
(c) 0.59  
(d) 0.5
- (ii) A strip footing is founded at a depth of 1.2m below the surface of a deep stratum of soft saturated clay having unit weight of 19.5kN/m<sup>3</sup> and unconfined compression strength of 50 kN/m<sup>2</sup>. The ultimate bearing capacity according to Terzaghi
- (a) 208.65 kN/m<sup>2</sup>  
(b) 350.22 kN/m<sup>2</sup>  
(c) 165.9 kN/m<sup>2</sup>  
(d) 198.4 kN/m<sup>2</sup>
- (iii) The maximum permissible total settlement recommended by IS: 1904 (1986) for a footing resting on plastic clay is
- (a) 40mm  
(b) 50mm  
(c) 75mm  
(d) 70mm

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- (iv) According to Coulomb's wedge theory, the active earth pressure slides the wedge
- down and outward on a slip surface
  - up and inward on a slip surface
  - horizontal upward and parallel to base
  - horizontal upward and parallel to base
- (v) When the soil mass is in active Rankine state, two sets of failure planes develop, each inclined at an angle  $\theta$  to the horizontal. The value  $\theta$  of is
- $65^\circ + \frac{\phi}{2}$
  - $65^\circ - \frac{\phi}{2}$
  - $45^\circ - \frac{\phi}{2}$
  - $45^\circ + \frac{\phi}{2}$
- (vi) A pile of 0.50 diameter and length 10m is embedded in a deposit of clay. The undrained strength parameters of the clay are cohesion = 60 kN/m<sup>2</sup> and the angle of internal friction is = 0°. The skin friction capacity (kN) of the pile for an adhesion factor of 0.6 is
- 671
  - 565.2
  - 283
  - 106
- (vii) When a retaining wall moves away from the back fill, the pressure exerted on the wall at the instant of full shear strength mobilization is termed as
- Passive earth pressure
  - Pore pressure
  - Active earth pressure
  - None
- (viii) Local shear failure will occur in soil when the  $\phi$  value is less than or equal to
- 29°
  - 36°
  - 40°
  - 42°
- (ix) A test plate 30cm square, settles by 10mm under a load of 9kN in a sand soil. A footing 1.5 m × 1.5m and subjected to a load of 225kN shall settle by
- 108 mm
  - 50 mm
  - 53 mm
  - 60 mm
- (x) The value of reduction factors  $R_{w1}$  and  $R_{w2}$  when the water table is at the base of the footing are
- 1.0 and 1.0
  - 0.5 and 1.0
  - 1.0 and 0.5
  - 0.5 and 0.5

2. (a) For the smooth retaining wall shown in the figure, make a sketch of the distribution of active earth pressure on the wall giving the principal values. Compute the total active resultant thrust per metre length of the wall. (5)



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- (b) Differentiate between Coulomb's earth pressure theory and Rankine's earth pressure theory. (4)
- (c) A retaining wall 6m high, has a smooth vertical back. The backfill surface is horizontal and in level with the top of the wall. The unit weight of the backfill soil is  $18\text{kN}/\text{m}^3$ , its angle of shearing resistance,  $\phi$  is  $30^\circ$  and the cohesion value,  $c$  is  $35\text{kN}/\text{m}^2$ . Determine the total active thrust per unit length of the retaining wall and its point of application. (6)
3. (a) A rectangular footing  $2\text{m} \times 3\text{m}$  is placed at a depth of 2m below the ground surface. The properties of the foundation soil are  $c = 25\text{kN}/\text{m}^2$ ,  $\phi = 36^\circ$ ,  $\gamma = 18\text{kN}/\text{m}^3$ . (8)

For  $\phi = 36^\circ$ ,  $N_c = 50.6$ ,  $N_q = 37.8$ ,  $N_\gamma = 56.5$ .

The load on the footing is concentric but it acts at an angle of  $15^\circ$  to the vertical.

Determine by IS 6403 (1981) recommendations, the net safe load that can be supported by the footing with a F.O.S. of 3 with respect to shear failure.

- (b) A strip footing 2m wide rests on the surface of a dry cohesionless soil having  $\phi = 17^\circ$  and  $\gamma = 18\text{kN}/\text{m}^3$ . If the flood causes the water table to rise temporarily to the surface, what percentage of ultimate bearing capacity is reduced? Take  $N_q = 5$ ,  $N_\gamma = 4$ . (2)
- (c) Loose to medium sand extends from ground surface upto a considerable depth. The water table is at depth of 3m from the ground surface. The N-value from standard penetration test at a depth of 5m is 18. Unit weight above the water table is  $18\text{kN}/\text{m}^3$  and saturated unit weight below the water table is  $20\text{kN}/\text{m}^3$ . Determine the corrected N- value of the soil. (5)
4. (a) A rectangular footing of size  $2.5\text{m} \times 3.5\text{m}$  is placed at a depth of 1m on a stiff saturated clay. The load acting on it is vertical but it is eccentric in both the  $x$  and  $y$  direction. The eccentricity is 0.2m in each directions. Compute the safe load on the footing if the factor of safety is three and the settlement is negligible. Take cohesion  $c_u = 105\text{kN}/\text{m}^2$ ,  $\phi_u = 0^\circ$  and  $\gamma = 17.8\text{kN}/\text{m}^3$ . Use I.S. code procedure. (6)
- (b) Elaborate with a neat sketch the procedure of conducting the seismic refraction test. (6)
- (c) What is a bore log? Explain it with the help of a neat sketch. (3)

5. (a) A pile group having nine piles were installed in a square pattern in a homogeneous clay deposit. The unconfined compressive strength of the clay deposit is  $140\text{kN/m}^2$ . The piles are 12m long and 400mm in diameter. The spacing between the piles are 900mm. Compute (3+4+2=9)
- The load carrying capacity of a single pile.
  - The load carrying capacity of the group pile.
  - The efficiency of the pile group.

Assume adhesion factor = 0.9.

- (b) Elaborate on the term negative skin friction. How can negative skin friction be determined for a pile group passing through a layer of clay of depth H which is susceptible to negative skin friction? (2+4=6)

6. (a) The results of a pile load test are given below. (3+6=9)

Load (kN)	Penetration (mm)
0	0
300	4.5
550	5.5
800	9.5
1050	14.5
1300	21.3
1550	30.0

The load test was carried out on a 300mm diameter pile having length of 10m. Draw the load settlement plot and estimate the allowable load of the pile as per Indian standard code of practice.

- (b) Slope stability analysis by the method of slices for a  $40^\circ$  slope on the critical slip plane gave the following results: (4)

Sum of the normal forces = 350kN; Sum of tangential forces = 200kN

Sum of neutral forces = 60kN; Effective angle of internal friction =  $35^\circ$

Effective cohesion =  $25\text{kN/m}^2$ ; Length of failure surface = 18m

Calculate the factor of safety with respect to shear strength.

- (c) Define the term stability number  $S_n$ . (2)

7. (a) An infinite slope in a  $c - \phi$  soil is inclined at  $12^\circ$  to the horizontal. The water table is at the surface and seepage is parallel to the slope.  $c = 10\text{kN/m}^2$ ,  $\phi = 25^\circ$  and  $\gamma_{sat} = 20\text{kN/m}^3$ . If a plane slip has developed at a depth of 5m, determine the factor of safety of the slope. (7)

- (b) What are the probable types of failure of a slope? (4)

- (c) Elaborate on the significance of the term Seat of settlement of a shallow foundation. (4)